LOST CANYON WATERSHED NONPOINT SOURCE POLLUTION CONTROL PLAN

Prepared for:

VILLAGE OF LAKE DELTON, WISCONSIN

INTEGRIT

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0 INTRODUCTION

1 BACKGROUND - PROJECT UNDERSTANDING

ne Lost Canyon Watershed is located within the Lake Delton Watershed in the southern p

the Village of Lake Delton. It extends along State Highway (STH) 12 from just no

velopments have been proposed for the Lost Canyon Watershed which will signif crease flooding and pollutant loading into the lake. This 1992 study began to address oblem of nonpoint source pollution from the Lost Canyon Watershed to Lake Delton. 1 of 1993, the Village of Lake Delton received a Lake Management Grant from the Wis epartment of Natural Resources (WDNR) to provide a holistic set of alternatives to

e study area for this project encompasses approximately 1,700 acres. Approximately this watershed is developed, and is projected to be 80% developed in the next 20 y ke Delton is already considered nutrient rich (eutrophic) and has experienced an alg ckweed nuisance for many years. Without stormwater management practices, this con Il only get worse. It is for this reason that the Village of Lake Delton applied for a anagement Grant in hopes of providing a plan to control nonpoint source pollution to the e overall goal for a Nonpoint Source Pollution Control Plan for the Lost Canyon Wat to contribute to the overall attainment of the resource of Lake Delton which include provement of natural habitat, water quality conditions, biological populations and recre 2. The Nonpoint Source Pollution Control Plan for the Lost Canyon Watershed will ev

improving the capacity of the system of storm sewers, swales, ditches, streams, et

control erosion, sedimentation and deposition in Lake Delton from the Lost C

reduce nonpoint source pollution loadings of nutrients, heavy metals and other pol

provide an implementation strategy and set priorities for the practices within the stud

ady Lane northward to the outlet at Lake Delton. Based on a 1992 lake management

is watershed is currently one of the greatest polluters to Lake Delton. A number of

2 PURPOSE AND SCOPE OF THE PLAN

aluate the nonpoint source pollution problems to Lake Delton.

make up the stormwater management system:

Watershed to minimize the need for dredging:

from the urbanized and developing areas of the watershed

i make recommendations to:

3 FORMAT OF THE PLAN

ne remainder of this document is presented in five major sections. A brief description ontent of each section is given below.

Section 2.0 Description of the Project Area

This section provides a general description of the physical parameters of the studincluding general location, drainage basins, soils, land use, and drainage patterns. included is existing regulatory ordinances.

Section 3.0: Methods and Assumptions for Determining Nonpoint Pollutant Loads

This section describes the steps used for assessing the present and predicted future no pollution potential of the project area. The base parameters and assumptions used Source Load And Management Model (SLAMM) are also described in this section.

Section 4.0: Methods and Assumptions for Determining Hydrologic Conditions

This section describes the tools used in analyzing the hydrologic conditions in the area. The hydrologic conditions are simulated on a computer model (SEDCAD various recurrence interval rainfall events for both existing and projected future conditions.)

Section 5.0: Recommended Design Criteria

This section outlines criteria for the proper management of stormwater in the Lost C Watershed.

Section 6.0: Storm Water Management Alternatives

This section views different Best Management Practice (BMP) alternatives and detheir effectiveness in alleviating drainage and water quality issues.

Section 7.0: Implementation Approach

0 DESCRIPTION OF THE PROJECT AREA

1 GENERAL LOCATION -

ne Lost Canyon Watershed is Located in the Village of Lake Delton and extends out

South-Central Wisconsin. The project area encompasses approximately 1,700 acainage area to Lost Canyon. Lost Canyon is a scenic 0.8 mile ravine cutting into the nd stone and conveys flow directly to Lake Delton. This canyon is also home to the

rporate limits into the Town of Lake Delton. The Village is located in north east Sauk (

anyon Tours, a horse-drawn carriage ride and Dell View Riding Stable and Stageconseback canyon tour. Both are popular tourist attraction.

2 DRAINAGE BASINS

or study purposes, the Lost Canyon Watershed was broken down into 15 sub-basins. b-basins range in area from 17 to 242 acres. Elevations within the project area vary out 1,020 feet above mean sea level in the southernmost sub-basin to 832 feet, a diff

out 1,020 feet above mean sea level in the southernmost sub-basin to 8 188 feet.

3 SOILS

ne soils in the study area classified in the Sauk County Soil Survey are predominately arms and loamy sands including the Plainfield loamy sands, Wyocena sandy loams chenry silty loams. Soils are well drained, medium textured underlain with sandy glacoutwash sand. Shallow or exposed bedrock conditions exist throughout the study are

ils are classified into hydrologic groups to indicate the minimum rate of infiltration ob bare soil after prolonged wetting. The soils in the watershed consist mostly of Hydroups A and B. Group A soils have low runoff potential and high infiltration rates (an 0.30 in/hr) even when thoroughly wetted. They consist of deep, well to excessively described the soils are classified into hydrologic groups to indicate the minimum rate of infiltration obtained by the soils in the watershed consist mostly of Hydrologic groups to indicate the minimum rate of infiltration obtained by the soils in the watershed consist mostly of Hydrologic groups to indicate the minimum rate of infiltration obtained by the soils in the watershed consist mostly of Hydrologic groups to indicate the minimum rate of infiltration obtained by the soils in the watershed consist mostly of Hydrologic groups and the soils in the watershed consist mostly of Hydrologic groups and the soils in the watershed consist mostly of Hydrologic groups are soils in the watershed consist mostly of Hydrologic groups are soils in the watershed consist mostly of Hydrologic groups are soils in the watershed consist mostly of Hydrologic groups are soils in the watershed consist mostly of Hydrologic groups are soils and the soils in the watershed consist mostly groups are soils and the soils are soils are soils and the soils are so

ids or gravels. Group B soils have moderate infiltration rates (0.15-0.30 in/hr) roughly wetted and consist chiefly of moderately deep to deep, moderately well to tined soils with moderately fine to moderately coarse textures.

LAND USE OF THE PROJECT AREA

sent day land uses within the watershed include commercial, low density residential trse, open space, woods, highway/roadway, agricultural and farmsteads. Areas were grocategories of relatively homogeneous impervious areas. A detailed delineation of lan

Total	total acres	81 %0	0% 232	601 %0	3% 28	101	33% 23	611	131	174	72% 71	7% 75	611	127	0% 128	
Highway/ Roadway	acres % to	0	0	0	-	S	œ	0	0	0	51 7	5	0	0	•	
Woods	% total	78%	25%	38%	40%	15%	%	18%	%!!	10%	8	21%	71%	72%	%	
»Mc	sacres	7	121	4	=	15	0	21	4	8	0	91	84	92	0	
Open Space / Undeveloped	fotal %	%/1	3%	79%	35%	63%	48%	767	%0	72%	78%	%09	%	2%	36%	
Open Space Undeveloped	acres	3	∞	28	9	8	=	35	0	126	20	45	0	3	20	
iltural	% total	%0	45%	%91	%	7%	%	797	%68	%	%	%	23%	%	36%	
Agricultural	acres	0	104	82	0	7	0	31	117	0	0	0	27	0	20	
Farmsteads	k total	%0	%	%0	%	% 0	%0	14%	%0	%	%	%	%	%0	%0	
Farms	acres	0	0	0	0	0	0	17	0	0	0	0	0	0	0	
rcail / iess	% total	%0	%	21%	22%	8%	%61	7%	%	17%	%	12%	%	12%	22%	
Commercail / Business	acres	0	0	23	9	8	4	∞	0	30	0	٥	0	15	78	

PROJECTED FUTURE LAND USE	

524

28% 28%

%%

	Total	
	/ay/ way	
	Highw Road	
	sp	
	Woo	
	bedol	
ID USE	Open S Undeve	
JRE LAN	rcall / ervious)	
ED FUT	Comme (50% Imp	
PROJECTED FUTURE LAND USE	rcall /	
	Comme Mixed	7 300 and 10 10 10 10 10 10 10 10 10 10 10 10 10
	Ajje	ŀ

Total	BCTOS	81	232	601	28	101	23	119	131	174	71	75	119	127
Way <i>l</i> Iway	% total	% 0	%0	%0	4%	%\$	35%	%0	%0	%0	73%	%9	%0	%0
Highway/ Roadway	acres	0	0	0	1	\$	∞	0	0	0	21	\$	0	0
Woods	% total	767	35%	34%	70%	22%	%0	2%	4%	%9	%	%9	%11	15%
Wo	acres	\$	82	38	9	22		~	\$	01		\$	13	61
Open Space / Undeveloped	100 %	%0 *	7%	34%	% S	%0	% 0	% 0	% 0	% 0	% 0	% 0	% 0	% 0
Open Space Undevelope	acres	7	5	37	1	0	0	0	0	0	0	0	0	0
ercal] / pervious)	acres % total	%0	0%	0%	%0	%0	%0	14%	%9	%8	%	797	48%	30%
Commercall / (50% Impervious	acres	0	0	0	0	0	0	91	6	14	0	19	88	38
Commercall / Mixed Use *	% total	%0	%0	787	%11	13%	%\$9	34%	% \$1	%8 E	27%	62%	%0 7	% 0
Commerca Mixed Use	acres	0	0	31	20	14	15	41	61	99	61	46	L Þ	0
Density intial	% total	31%	62%	3%	%0	%	%0	47%	75%	43%	%0	%0	%!	%
Medilum Densi Residential	acres	9	145	4	0	0	0	9\$	86	74	0	0	-	0

acres	18	232	188	28	0	23	119	131	174	71	75	119	127	
% total	%0	%0	%0	4%	2%	35%	%	%0	%	73%	%9	%0	%	
acres	0	0	0	-	5	∞	0	0	0	15	\$	0	0	
% total	767	35%	34%	70%	757	%0	2%	4%	%9	% 0	%9	%11	% \$1	
acres	\$	82	38	9	22		\$	\$	01		\$	13	61	
total	40%	2%	34%	5%	%	%0	%0	%0	%0	%0	%0	%0	%0	

lage's projected annexation plan. Table 1 also lists the projected future land uses within in. CURVE NUMBER (CN)

e SCS has evolved a system of curve numbers to estimate runoff production potential.

nbers are based on soil permeability, land use (cover types) and antecedent mo ditions. Table 2 provides composite CNs for each subbasin.

WATER RESOURCE CONDITIONS / DRAINAGE PATTERNS Lost Canyon Watershed does not contain any surface waters and has no perennial str

rmwater comprises all of the flow through the canyon, which can be very significan vy rain. As rain falls on roads, buildings, lawns, etc., the stormwater picks up sedimen er pollutants from the watershed and deposits them directly into the lake. The dra terns delineated in Figure 1 are broken down and described below:

Basin 15 (246 ac)

This drainage basin collects stormwater along a flat, heavily vegetated swale intermittent stream flowing to the north to Fern Dell Road, east of STH 12.

This basin contributes to the same intermittent stream which began in Basin 15. The s

flows north from Fern Dell Road and east of STH 12 northwesterly towards the I-

This drainage basin is entirely contained in the southwest quadrant of the I-90/94 - ST Interchange. Drainage pattern flows northerly through the Yellow Thunder Subdivisio

continues underneath I-90/94. Basin 12 (119 ac)

Basin 14 (128 ac)

Basin 13 (127 ac)

and STH 12 Interchange.

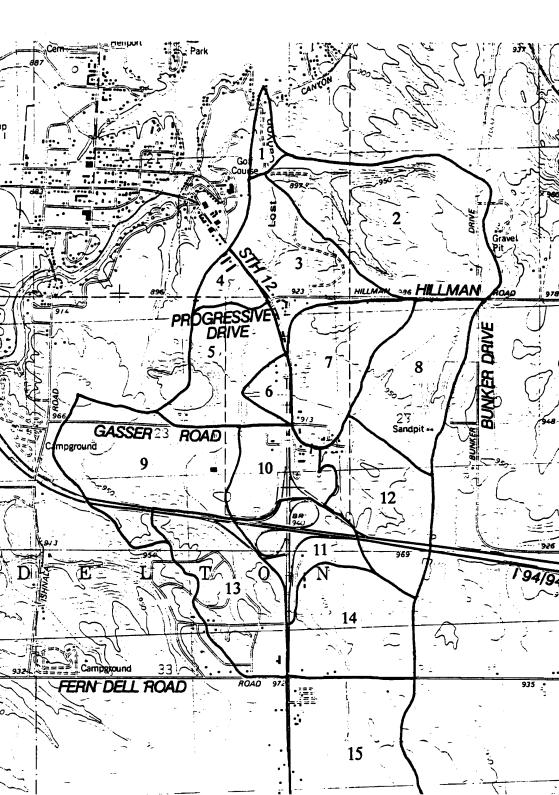
This basin drains an area in the northeast quadrant of the I-90/94 - STH 12 Interch The intermittent stream draining Basin 12 flows in a northeasterly direction through heavily vegetated area

- Table 2

Hydrologic Input Parameters

Lost Canyon Watershed

Sub-basin	Area (acres)	Curve Number (Existing)	Curve Number (Future)	Time of Concentration (Hours)
1	18	61	70	0.17
2	232	60	66	0.65
3	109	65	71	0.63
4	28	72	86	0.25
5	101	72	85	0.61
6	23	71	91	0.24
7	119	71	85	0.51
8	131	71	81	0.54
9	174	71	81	0.44
10	71	60	68	0.54
- 11	75	60	81	0.54
12_	119	52	81	0.57
13	127	46	58	0.92
14	128	64	81	0.75
15	246	57	69	1.5



Basin 10 (71 ac)

This basin drains an area east of Highway 12 just north of the I-90/94 - STH Interch

Basin 9 (174 ac)

This basin drains an area on the north side of I-90/94 east of Basin 10 including portion of the Wisconsin Dells Greyhound Racing Park. This intermittent stream me to the north where it joins with the flows from Basin 10 at Gasser Road.

Basin 8 (131 ac)

This drainage basin collects water from a largely undeveloped area with a fairly stee (4 percent), dropping over 80 feet in less than one half of one mile. This is the on which drains primarily to the southwest to meet up with a northerly flow from Bas

Flows from the southeast (Basin 12) and northeast (Basin 8) converge and continue

Basin 7 (119 ac)

a flat stretch through agricultural fields west towards STH 12.

Basin 6 (23 ac)

This slightly urbanized basin is one of the smallest in the watershed. The basin of stormwater runoff from STH 12 adding to it the flows from Basin 7.

Basin 5 (101 ac)

Drainage patterns involve the convergence of several different flows into and throu Village's largely undeveloped commercial lands and business park. The basin has sl steep slopes which channel runoff to the intermittent stream meandering almost due

Basin 4 (28 ac)

Located between Progressive Drive and STH 12, Basin 4 is the second smallest basin watershed. The basin has moderate slopes flowing northeast towards and under ST.

Basin 3 (109 ac)

Basin 1 (18 ac)

low.

This is the northernmost portion of the Lost Canyon watershed. This small basin is with steep picturesque canyon slopes which takes water from the south and east thro canyon to a bay at the south end of Lake Delton.

.7 TIME OF CONCENTRATION (TC)

he time of concentration is an important parameter in estimating peak rates of stormoff. The shorter the time of concentration a basin has, the greater the peak runo ime of concentrations used in the hydrologic modeling were presented in Table 2.

8 EXISTING REGULATORY REVIEW

he Village of Lake Delton currently has an enforceable erosion control ordinance (Ordinance of So. 9-91-273). The ordinance follows the model construction site erosion control or utlined in the Wisconsin Construction Site Best Management Practice Handbook. By fast the Sediment in the Lost Canyon Watershed comes from constructions sites.

- lanagement Plan and stormwater drainage facilities for any development with land disctivity of over 20,000 square feet. No final subdivision plat can be approved and no be ermit can be issued until and unless a Stormwater Management Plan has been review approved by the Zoning Administrator. Each project is individually reviewed for comith regulations. Some of the key criteria of the stormwater management ordinance are
 - The controlled release rate of stormwater runoff from all developments describe this section shall not exceed that of the pre-development or undisturbed condition on a 10-year, 24-hour storm.
 - Streets, blocks, lots, parks, and other public grounds shall be located and laid such a manner as to minimize the velocity of overland flow and allow ma opportunity for infiltration of stormwater into the ground.
 - Detention facilities shall release stormwater at a non-erosive velocity.

0 METHOD AND ASSUMPTIONS FOR DETERMINING NONPOINT POLLUTANT LOADS

1 BACKGROUND INFORMATION

onpoint pollution loadings from the Lost Canyon Watershed were determined usi

isconsin Department of Natural Resources (WDNR) "Source Loading and Manage

hieve maximum reduction of the pollutant loadings to Lake Delton. Pollutants of inte s study are particulate sediments, nutrients (measured as particulate phosphorous) and etals (measured as total lead). The model calculates loadings based on several

tracteristics of the watershed being placed on one master map file including existing ure land uses, soil hydrologic grouping, and drainage basins. The second is to create SLAMM data files specifically tailored to each land use in the Lost Canyon Watershe

description of all existing and future land use types within the watershed can be for ole 1 of Section 2.0. Both existing and future land use delineations were placed on a

odel" (SLAMM). This model was developed by and for the WDNR to be used in iority Watershed - Nonpoint Source Pollution Abatement Program. SLAMM is u

lculate the annual pollutant loadings for each basin in the Lost Canyon Watershed. adings are then used to prioritize 'critical' areas by loading rates in tons or pounds per lese results help to target areas where Best Management Practices (BMP's) can be pla

Drainage System

Existing and Future Land Uses

Existing Drainage Control Practices

Soils Hydrologic Grouping

:luding:

- Annual Rainfall
- Street and Roadway Conditions
- PROCEDURE FOR POLLUTANT LOADING ESTIMATION
- llutant loading estimation relies on two processes. The first consists of all of

a master map for the Lost Canyon Watershed.

Each drainage basin in the study area was then divided into land parcels of varying six on 1) hydrologic soil type (A/B or C/D) and 2) land use characteristics. LAMM data files rely on a number of sub-files to compute pollutant loadings. F

ainfall, runoff source areas and pollutant data were selected to best represent site conditi ollutant characteristics of the watershed to predict annual pollutant loading rates. Rain: or 1982 was used in the SLAMM model as suggested by the WDNR because it is cor o represent a typical year of rainfall.

each land use was broken down by contributing source areas such as streets, parking lots andscaped areas and so on. Source areas were further refined to reflect the percent of onnected impervious areas (DCIA) where needed. These are source areas which drain r less directly to the streets. All other areas are considered to be non-DCIA and typical grassed areas. Street parking density, roadway drainage type (swales, curbs, etc.) a treet sweeping practices were also considered. All of the above information was compi set of files created for both sandy (A/B) soil types and clayey (E/D) soil types un ossible land uses. Il of the data files were run using SLAMM Version 6.1 to analyze for particulate sediments), particulate phosphorous, and total lead loadings. The model reports the res ediments in tons per year and phosphorus and lead in pounds per year. These calculati

hese results were then tabulated and added to the watershed parcel spreadsheet. A rout reated to calculate loadings where criteria matched for each subbasin based on its ind paracteristics. These loadings were then summed by subbasin to assess current and nticipated loading rates. ediment loadings to Lake Delton were also calculated using RUSLE (Revised University oss Equation) which computed erosion and sedimentation based on flow velocities and plumes (i.e. runoff energy). This method was used with the watershed model SEDCAI

ported as statistical average pollutant loadings for a typical year of rainfall.

3 NONPOINT SOURCE IMPACTS

; rain falls on rooftops, roads, lawns and parking lots, stormwater runoff picks up po ch as sediments, nutrients, pesticides, road salt, and bacteria. These pollutants can con

increased turbidity, algae blooms, macrophyte growths and unsafe swimming conditi

ed the actual erodibility of soil, slope of the basin, land cover and flow length to impute erosion and sedimentation rates for existing areas with a more rural setting.

oncentrations may reach the point where they can be toxic or a health hazard to human quatic organisms. 4 ESTIMATED POLLUTION LOADINGS

stimated pollutant loadings for each subbasin are summarized in Table 3. Nearly hal onpoint source pollutants originate in three Subbasins - 3, - 9 and - 14. In each of

ibbasins commercial property makes up 17 percent to 22 percent of the total area (Ta abbasin 6 has the highest rate of pollutant loadings. This is due to the high percent bbasin which is either commercially developed and in Highway 12. Subbasins with high

pollutant loadings which are considered critical are Subbasins 3, 4, 6, 9, 11, 13 Figure 2). These subbasins have estimated loading rates which are above the average atershed. Average pollutant loading rates for the Lost Canyon Watershed would be con gnificant when compared to other urban watersheds.

otal pollutant loadings and loading rates are expected to increase dramatically as the wa evelops. Table 4 summarizes predicted future nonpoint source pollutant loadings. Su hich are not currently critical sources of nonpoint source pollutants will become cri evelopment occurs (Figure 3). Figure 4 shows the most critical increases in nonpoint ollutant loadings which are highlighted in Table 5. This increase in nonpoint source pe adings will have a noticeable and adverse impact on the water quality in Lake

nerefore, it is important that proper Best Management Practices are constructed for

isting and future development. 5 BACTERIA IN STORMWATER RUNOFF

PA water quality standard, which is 200 fecal coliforms per 100 ml or less, duri

mediately after storm events. This is particularly a problem in lakes and slow-moving d streams. In highly publicized events during 1993 and 1994, stormwater runoff was high bacteria levels that resulted in beach closings and threaten drinking water supplie

mples taken by the U.S. Environmental Protection Agency (EPA) and the Wisconsis dicate that bacteria levels in stormwater runoff often cause surface water bodies to exc

e coast of Lake Michigan from Sheboygan to Kenosha.

e presence of bacteria in stormwater runoff in itself may not be a concern. However ed as an indicator of the potential presence of pathogens that would be a risk to human cteria is present anywhere there are warm blooded animals, both domestic and wild

nere there is decomposing organic matter. Of most concern are sources which contain d

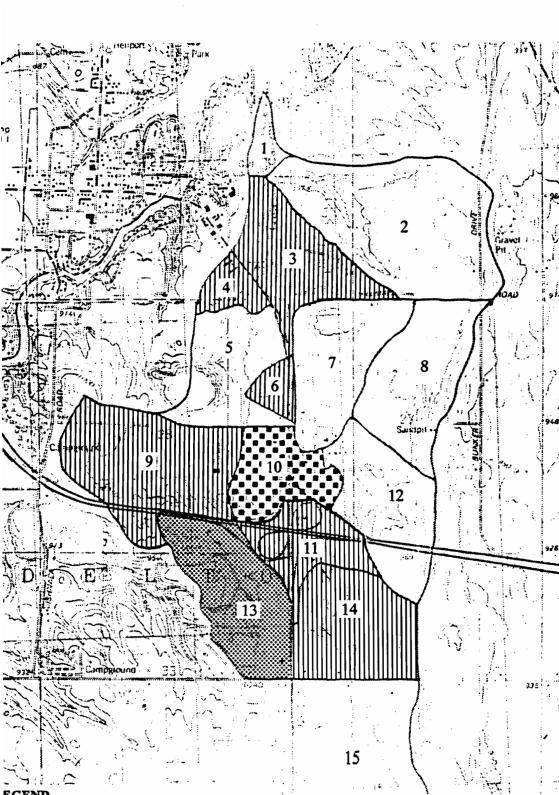
is difficult to estimate, with any reliability, the bacteria levels that will be in storaged in the variability of site specific conditions and the weather. Sampling eriod of time is the only way to identify and quantify a problem. Sampling and to atside the scope of this study, therefore, this investigation will focus on estimated actions of sediments, heavy metals and phosphorous (nutrients). However, recommendation reducing nonpoint source pollution in the Lost Canyon Watershed will also help

acteria levels in the stormwater runoff from the Lost Canyon Watershed.

CRITICAL BASIN IDENTIFICATION

Existing Non-Point Source Pollutant Loadings

		POLLUTANT	TANT			
	Lead	ad	Zinc)C	Phosphorous	orous
_	(lbs/year)	lbs/ac/yr	(lbs/year)	lbs/ac/yr	(lbs/year)	lbs/ac/yr
80	0.1	0.003	0.1	0.003	0.1	0.007
2.80	0.7	0.003	0.7	0.003	9.1	0.007
	0.09	0.549	33.6	0.308	47.8	0.438
40	18.3	6593	11.5	0.410	15.3	0.547
17.66	34.0	0.336	20.4	0.201	28.2	0.278
96	30.0	1.306	20.0	0.868	24.7	1.074
	23.6	861.0	13.1	0.110	20.1	0.169
3.88	0.5	0.004	9.0	0.005	1.4	0.010
	80.3	0.461	41.5	0.238	62.8	196.0
	42.8	09'0	29.4	0.414	34.8	0.491
136.36	32.0	0.425	17.3	0.229	24.9	0.331
	2.1	810'0	1.8	0.015	3.7	0.031
	44.6	0.351	23.2	0.183	37.7	0.297
227.09	75.4	685'0	48.8	0.381	64.9	0.507
7.25	3.7	0.015	2.6	0.011	4.1	0.017
5	448.1	5.515	264.4	3.378	372.1	4.564
80		896.0		0.225		0.304



CRITICAL BASIN IDENTIFICATION Future Non-Point Source Pollutant Loadings

		POLLUTANT	TANT			
	Lead	pe	Zinc	20	Phosphorous	iorous
lbs/ac/yr	(lbs/year)	lbs/ac/yr	(lbs/year)	lbs/ac/yr	(lbs/year)	lbs/ac/yr
37.59	1.3	0.073	1.5	0.081	2.2	0.122
72.59	33.2	0.143	36.9	0.159	55.1	0.237
283.36	83.2	0.762	45.7	0.418	0.99	0.604
716.67	\$5.5	1861	31.6	1.129	44.0	1.573
730.92	208.1	2.056	111.5	1.101	161.1	1.592
761.86	58.7	2.554	35.6	1.548	46.8	2.037
471.81	148.3	1.246	81.1	0.682	124.1	1,043
237.22	76.4	0.583	50.0	0.382	17.6	0.593
470.43	216.3	1.243	119.3	0.686	181.1	1.041
251.89	74.8	1.053	43.0	909'0	57.7	0.813
752.51	158.2	2.101	74.0	0.983	117.0	1.553
623.88	204.9	1.722	8.06	0.763	149.5	1.256
222.70	75.3	0.593	33.7	0.265	61.2	0.482
841.60	299.2	2.337	127.8	666.0	215.0	1,680
141.35	87.4	0.355	41.4	0.168	83.7	0.340
6,616.40	1780.6	18.801	924.1	026'6	1442.3	14.965
441.09		1.253		599'0		866.0

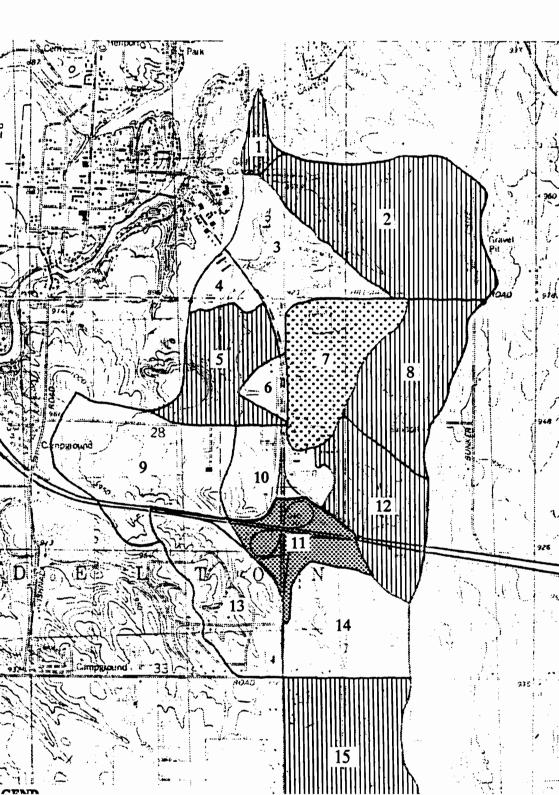
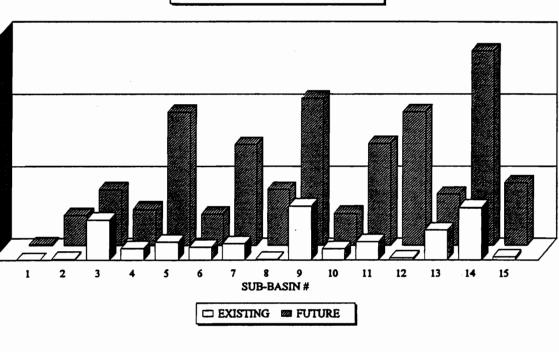


Figure 4
Sediment and Phosphorous Loadings

SEDIMENT LOADING EXISTING AND FUTURE LAND USES



PHOSPHOROUS LOADING EXISTING AND FUTURE LAND USES

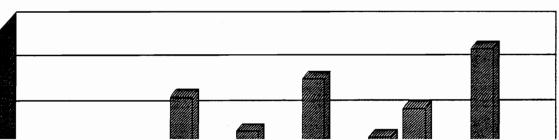
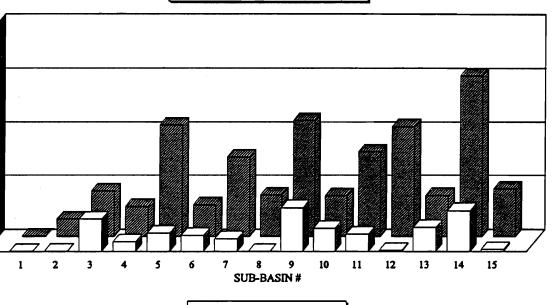


Figure 4 (cont.)
Lead and Zinc Loadings

LEAD LOADING EXISTING AND FUTURE LAND USES



□ EXISTING **■** FUTURE

ZINC LOADING EXISTING AND FUTURE LAND USES

FUTURE NON-POINT SOURCE POLLUTANT LOADINGS Projected Increase Over Present Loading Rates

					POLLUTANT	TANT					
	Sediment			Lead			Zinc			Phosphorous	
	lbs/ac/yr	% Increase	(lbs/year)	lbs/ac/yr	% Increase	(Ibs/year)	lbs/ac/yr	% Increase	(lbs/year)	lbs/ac/yr	% Increase
╙	37.59	1342%	1.3	0.073	2423%	1.5	0.081	3693%	2.2	0.122	1740%
L	72.59	7283%	33.2	0.143	41924	36.9	0.159	\$305%	\$5.1	0.237	3390%
	283.36	138%	83.2	0.762	%6E1	45.7	0.418	136%	0.99	0.604	138%
	716.67	307%	5.5.5	1.981	304%	31.6	1.129	276%	44.0	1.573	288%
	730.92	733%	208.1	2.056	%219	111.5	101'1	\$47%	1.191	1.592	\$72%
	761.86	244%	2.85	2.554	%561	35.6	1.548	178%	46.8	2.037	%061
<u> </u>	471.81	%519	148.3	1.246	%8 <i>2</i> 9%	81.1	0.682	%619	124.1	1.043	2819
_	237.22	%0 2 19%	76.4	0.583	14675%	50.0	0.382	8374%	17.6	0.593	5749%
_	470.43	273%	216.3	1.243	769%	119.3	0.686	288%	181.1	1.041	288%
_	251.89	279%	74.8	1.053	175%	43.0	909'0	147%	27.7	0.813	%991
	752.51	\$25%	158.2	2.101	495%	74.0	0.983	428%	117.0	1.553	410%
_	623.88	%1709 **	204.9	1.722	%187%	8.06	0.763	4962%	149.5	1.256	%950
-	222.70	%691	75.3	0.593	169%	33.7	0.265	145%	61.2	0.482	162%
_	841.60	371%	299.7	2.337	397%	127.8	0.999	262%	215.0	1.680	332%
	141.35	1950%	87.4	0.355	2390%	41.4	0.168	1599%	83.7	0.340	2024%
-	6,616.40		1780.6	108.81		924.1	9.970		1442.3	14.965	
\vdash	441.09			1.253			0.665			0.998	
ı											

basins where the pollutant loadings increase more than five times (500%) that of existing (1994) conditions.

0 METHOD AND ASSUMPTIONS FOR DETERMINING **HYDROLOGIC - HYDRAULIC CONDITIONS**

1 BACKGROUND INFORMATION

ne runoff and sedimentology model SEDCAD+ was used to analyze the hydrologic/hy-

Existing Conditions

debris causing flooding upstream.

through Kennel Club and under Gasser Road).

nditions of the project area. SEDCAD+ was developed by the University of Kennick

ws, SEDCAD + determined peak stormwater discharges throughout the drainage system out data includes area, soil types, and flow path information as described in Section 2

e schematic of the SEDCAD+ model in Figure 5 shows the order in which the ocessed the calculations. The model has the capability to determine peak runoff rates a

sults of the modeling and historical records reveals drainage concerns within the study e most common reason for flooding in the Lost Canyon Watershed is simply that culve dersized and are unable to convey the flows cause stormwater to back up and flood ac operty. The Lost Canyon conveyance system was analyzed for its ability to convey m the 2, 10, 25, 50 and 100-year recurrence intervals. Estimated flows at selected los oughout the study area are summarized in Table 6. Existing and future deficiencies

The private culvert (42" CMP) in Basin 4 leading to a 6' x 4' concrete box of under STH 12 just north of Progressive drive is undersized and is obstructed

Culvert under old service road in Basin 10 directly north of interchange has coll This temporarily offers some detention and relief on downstream structures (Co

Culvert (36" CMP) between Kennel Club Inn and Suites and Wisconsin Considered Design Design and train 20% at 20% and trains agree to the Control Chi

DCAD + simulated hydrographs for each basin. subwatersheds and combing and routing

assist in design of storm water conveyance and detention facilities.

mputes both runoff volumes, flows and sediment loadings. The program was also dev

provide design criteria for Best Management Practices (BMPs).

2 CURRENT HYDROLOGIC-HYDRAULIC CONDITIONS

rmwater management system are described in the following sections.

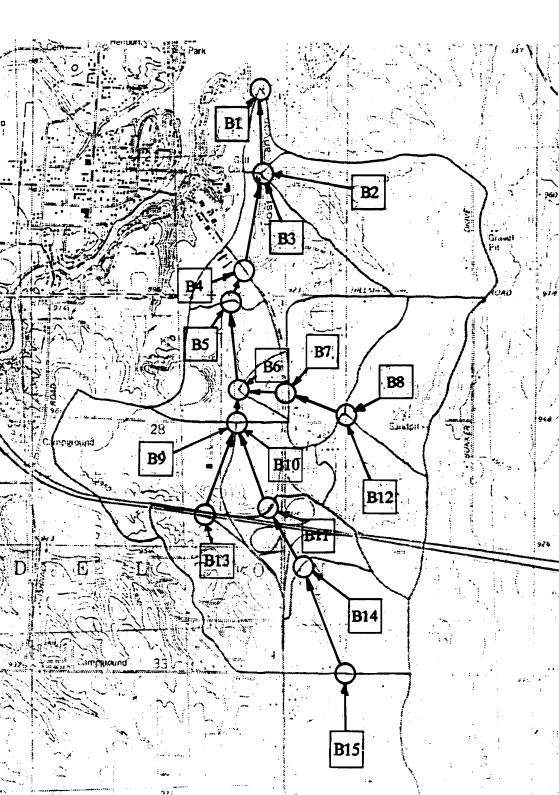


Table 6

CALCULATED FLOWS FOR LOCATIONS IN THE LOST CANYON WATERSHED

Existing Conditions

		Return Period			
Location	Contributing	2-уеаг	10-year	25-year	50-year
	Watersheds	(cfs)	(cfs)	(cfs)	(cfs)
Lake Delton	1-15	80	205	350	420
tream of Progressive Drive	5-15	90	310	400	525
vnstream of Interchange	11, 14, 15	15	75	105	140
under HWY 12 North of Gasser Rd.	7, 8, 12	65	205	256	340

Proposed Future Conditions

		Return Period			
Location	Contributing	2-year	10-year	25-year	50-year
	Watersheds	(cfs)	(cfs)	(cfs)	(cfs)
Lake Delton	1-15	265	455	605	850
ream of Progressive Drive	5-15	325	700	840	1,015
nstream of Interchange	11, 14, 15	80	205	240	300
under HWY 12 North of Gasser Rd.	7, 8, 12	220	465	550	670

Culvert (60" CMP) under STH 12 (Basin 7) would be exceeded for the 50 and 10 events.

Future Conditions

All concerns listed in the existing conditions above apply including:

- Existing Detention facility just south of Progressive drive is undersized
- Increased flows are expected to create scouring and erosion throughout the cany

RECOMMENDED DESIGN CRITERIA

NTRODUCTION

: is a growing awareness that land use decisions affect stormwater management needs water management needs and decisions, in turn, affect land use decisions. Cu

water management studies, such as this one and the Spring Brook Bay Watershed Plan rtant first steps to addressing the stormwater management needs of these two a

lopments and planned developments south of Progressive Drive, along both side way 12, have helped to focus on the specific needs of these areas. Highlighted ar to provide safe passage for both large and small storms to protect the environment a

ort the quality of life for commercial, residential, and tourist trade of Lake Deltor rehensive stormwater management system is necessary to convey stormwater safely thr ost Canyon Watershed to Lake Delton. The selected stormwater management system nmodate existing and future commercial, residential, and recreational interests in the

ent manner possible and with minimum disruption to commercial interests of this ar i, often unplanned, peripheral development has frequently been the source of n icts with long established waterways. In watersheds, like Lost Canyon, that do not nial streams, development tends to forget to plan for the inevitable flooding that will of

ings are often constructed too close to the waterway or constructed across the water ng safe passage and future capacity expansions difficult and costly. Once areas have , or even partially developed, there is often little that can be done to alleviate ems. However, toward the peripheries of the developed areas, where development pret beginning, it is still possible to influence where development occurs, how close it will iting stormwater passage areas, and the type of service that will be required to

ng and future facilities. ptimize the level of control or services provided, recommendations incorporate tment Train" that is depicted in Figure 8-1. The concept of the Treatment Train incl fore simple less costly source controls as well as the more traditional structural cont

proposed recommendations will enable the Village of Lake Delton to protect the v y in Lake Delton by reducing nonpoint source pollutant loadings in stormwater runo

SOURCE CONTROLS called "Housekeeping Measures", source controls are procedures or activities that pro-

luce the amount of stormwater coming into contact with potential pollutant sources. So ols for the Village of Lake Delton range from informing the public on proper urcing erosion control standards. The Uniform Dwelling Code (UDC), established R 20-25 and ILHR 50-25 and WDNR's NR-216 require erosion control measure and the control of t

truction sites. Village inspectors are currently enforcing State erosion control stand nter-agency agreement is being developed which will clarify the rolls of the City, W. DILHR. It is estimated that the control of erosion from construction site should report the rolls of the City, W. Diller. It is estimated that the control of erosion from construction site should report the rolls of the City, W. Diller. It is estimated that the control of erosion from construction site should report the rolls of the City, W. Diller. It is estimated that the control of erosion from construction site should report the rolls of the City, W. Diller. It is estimated that the control of erosion from construction site should report the rolls of the City. W. Diller. It is estimated that the control of erosion from construction site should report the rolls of the City. W. Diller. It is estimated that the control of erosion from construction site should report the rolls of the City. W. Diller. It is estimated that the control of erosion from construction site should report the rolls of the City. W. Diller. It is estimated that the control of erosion from construction site should report the rolls of the rol

void the expense of correcting future stormwater management problems, the Village sate its stormwater management ordinance. The ordinance should require standard ess both the quantity and quality of stormwater runoff. The Village could use the

5.2.2 Stormwater Management Ordinance

el ordinance being developed by the DNR for stormwater management. The ordinance ify which projects would need to get a stormwater permit, would there be any project ld be exempted from needing to get a stormwater permit, what information would ired to be submitted, and who may prepare the information submitted in the prication (i.e. does the permit have to be prepared by a Registered Professional Engineers, for violation of the ordinance. One of the most important policy decision which so included is the issue of maintenance. Maintenance of stormwater facilities is often the dedication of a maintenance easement; establishing who legal responsibility to preputial maintain any structures; a maintenance plan and per

5.2.3 Public Education

not properly maintaining the facilities.

objective of public education is to make individuals aware of the problems cause mwater runoff and the measures that they can take to minimize the harmful efficiences must first be identified. There are at least three audiences in the Village of on, the business community, the general public, and school age children. A take gage should be developed for each audience.

City should consider developing basic informational materials such as, brochures, dispersions, etc., that are specific to the Village of Lake Delton's stormwater manageram. It may include the preparation of articles for the local news paper or the Village

mational signs along area water ways would compliment the stenciling. Signs could ed public about water quality issues. Topics which could be included are nonpoint s tion sources, causes for and general information about the fish advisory, citizen poll ention techniques, and recognition of the implementation of best management practic

ic education program of the Village's recycling program will benefit the Vill iwater program by providing instruction for the proper disposal of vehicle oil, antifr izers, pesticides, old paint and other house hold hazardous wastes that often fine their area water ways.

Village would be encouraged to develop a comprehensive program targeting school Iren. This approach has been very successful in getting participation in the recy rams. In addition to classroom materials, audio visual materials could also be developed rtable, interpretive water quality display could be part of the comprehensive program

5.2.4 Maintenance

ine maintenance cleaning of streets, ditches, swales, and storm sewers is essentia cing the amount of pollutants accumulated on Village streets, ditches and sewers that ashed into area water ways. The Village does not adequately clean streets and ditch

ficantly reduce nonpoint source pollution. Site specific improvements to the Vill ation/maintenance program include, weekly sweeping of Village streets and parking le nercial areas and routine cleaning of ditches and storm sewers on a three to four year of

5.2.5 Spills and Illicit Connections

rdous materials. The review should evaluate whether adequate measures would be prevented pollution of storm sewers ditches and Lake Delton. Village should establish an inspection program of all drainage facilities to locate

Village should review established procedures for responding to the spill of poten

nate all non-stormwater discharges which are not properly permitted.

AREA CONTROLS

rcement of the Village's Erosion Control and stormwater management Ordinances w oundation of the area controls implemented by the Village. It has been estimate ous investigations that erosion from construction sites has historically contributed over

a total sediment loadings to Wisconsin's rivers and lakes. Enforcement of the er

ow swales. The system should be designed to treat the "First Flush" of pollutants waring the first few minutes of a rain event. The ":First Flush" typically refers to pollutined in the runoff from the first 1.5 inches of rainfall. The combination of check dam tation would slow stormwater runoff and allow particles to settle and be filtered betation. Area controls would be effective, inexpensive practice that could be incorporate.

include a series of check dams constructed to slow stormwater runoff flowing the

the site development in areas such as the Village's Business park.

development should be required to construct stormwater management facilities to hacreases in runoff rates, volumes and pollutant loadings. Criteria and standards should

I to existing building codes that require new develop to construct on-site facilities to First Flush" (runoff from the first 1.5" rainfall) and contain the peak runoff rate for the and 100-year storm events to predevelopment conditions. Structural controls could a swales and check dams to on-site detention ponds. The controls will ensure lopment pays its own way. Village residents will therefore, avoid paying for rebuiling Village facilities to accommodate development. However, as an alternative to require

ite facilities, the Village could collect a fee or an assessment from new developmen

DEVELOPMENT AND REGIONAL CONTROLS

d pay for their share of over-sized facilities.

proposed stormwater management ordinance should not only apply to the development idual sites but should also apply to larger new developments and existing development, residential subdivision or the Village's business park. Development and regional concuse use of the economy of scale to optimize on-site area controls and larger system concurred recommended that large developments such as the business park require

ant"/property owner to landscape their properties to detain the "First Flush". The orbities would help contain spills and would provide partial treatment of stormwater rublection systems of swales and storm sewers would be designed to convey runce egically placed regional detention ponds. Detention ponds should be designed to respond to percent of sediments contained in stormwater runoff. Detention ponds should need as a landscaped water features that improves stormwater quantity and provides

ection, similar to the detention area north of Progressive Drive. Detention ponds she ever possible, be designed as off-line facilities so that they are unaffected large s.

Onal controls are strategically located and sized to achieve flooding and water quality

ighout the watershed. Regional facilities can offer an economy-of-scale for reduction flood protection reduction of maintenance, and increase system reliability

rge groundwater supplies. Infiltration facilities should be designed with a safety fact 2) to account for clogging by the accumulation of fine soil particles (silts and clays). d be designed to drain within a 72 hour period (the average time between storms even bottom of the infiltration facility and the top of bed rock or the top of the water table signed.

parated by at least three feet of soil. Keeping infiltration basins vegetated helps to experiormance of the facilities. Infiltration should be routinely cleaned to make ability and remove pollutants. Strict procedures to control spills of potentially hazar

ration is a practice that is most effectively used as an Area Control of nonpoint s

rials would have to be enforced by the Village in areas where infiltration is used.

CORRIDOR PRESERVATION

lanning and development. Its goal is to prohibit, or at least minimize, development in are likely to be required to meet stormwater management needs in the future. These de: lands adjacent to existing stormwater conveyance areas which are projected to recity expansion; areas which might be needed to construct entirely new channels or converve neighborhoods or commercial developments; and land needed for stormwation/detention facilities.

n corridors are preserved in advance, negative land use and social impacts, as well a

dor preservation is one means of coordinating stormwater management planning with

of stormwater management improvements, are minimized. However, when land in rved for future needs, disruption of residences and businesses is a frequent result, an of obtaining the land and/or constructing alternate stormwater conveyance facilities inwater detention facilities is very likely to be considerably higher. At times, the new overment cannot even be made because the disruption and cost would be too great. Sidor preservation should be undertaken now. Public opinion (private and public second studies, and the Village's efforts to deal with ongoing development issues and con

stormwater management all intensely focus the need for additional or expanded stormwater facilities. Planned development within the watershed makes it mandatory to fe, efficient, and economical means of managing stormwater in this area. It is espectant in areas such as the Lost Canyon Watershed south of Progressive Drive on bot

nd west side of Highway 12, as well as in the Spring Brook Watershed, and the water ing through the Pizza Pub area.

6.6.1 Problems with Implementing Corridor Preservation

go a long way toward establishing corridors and detention/retention facility sitesing stormwater management—land use conflicts.

5.6.2 Methods of Corridor Preservation

.2 Methods of Coffidor Freservation

Purchase of Land and Interest in Land - The outright purchase of land for future us be done where there is no question as to the final facility location.

e are several techniques that can be used to preserve corridors for future storm

Official Mapping - A technique short of land acquisition is the official mapping of a stormwater management corridors and sites. When planning, the Village has the state authority to prepare plans and maps showing the approximate location and size of a stormwater corridors and detention/retention facilities. The purpose of the map is to in the public and land developers which areas may be required for future conveyance.

Developer Dedications - It would be appropriate for the Village to build into subdivision ordinance the right to require developers and individuals to dedicate a poof their land for necessary stormwater management improvements. This technique is

to acquire land for the construction of stormwater conveyance facilities, as well as to on the needed right-of-way for facility expansions. Dedications or payment in lie construction could then be used to require developers to set aside land in appropriate a cor if a regional solution is desired, payment in lieu of a land set aside so that a regional which might be constructed more efficiently could be constructed to resolve if or the area.

Village of Lake Delton's role

Village of Lake Delton should develop a corridor preservation master plan after evaluated for corridor preservation. Indicated in the plan would be the type of improver used and when, and the plan should propose a method of monitoring future developed to the extent possible. Specification of the extent possible.

Be an advocate for corridor preservation efforts;

Daise the issue with local developers and landowners

5.6.3

lillage should:

- What new stormwater management facilities will be needed? Where is it possible to keep development out of the right-of-way needed for f ,
- stormwater management purposes? Develop a map showing those areas where preservation efforts should be undert •
- Monitor development pressure and give priority for corridor preservation effo those areas where development pressure is the greatest.

Develop corridor preservation strategies:

Where it is determined that future stormwater management expansions will be nece and where corridor preservation is still possible, map the needs and work extraterritorial government officials seeking better cooperation.

Monitor and document the progress of preservation efforts.

FLOOD CONTROL CRITERIA

atersheds, like Lost Canyon, that do not have perennial streams, development patterns rget to plan for the inevitable disaster that a flood will bring. Previous discussion

ken blue line (intermittent stream).

nportance of preserving the natural drainage corridors. it is recommended that these na

age corridors, sometimes referred to as the "Primary Drainage System", be sized to co or store the runoff from a 100-year storm event. The "Primary Drainage System" i Canyon Watershed is delineated in Figure 1 on the USGS 7 1/2 minute quadrangle ma

e are three constraints in the primary system, the 42" private culvert leading to High orth of Progressive Drive, the 60" east-west culvert under Highway 12 north of G and the culverts under the I-90/94 Interchange. It is recommended that regional dete s should be constructed at each of these locations to attenuate flood flows to the capa

e existing culverts. DRAINAGE

stormwater collection system of swales, culverts, and storm sewers from subdivision idual developments should be sized to handle the 10-year storm. The Village's exi

water regulations are for the most part adequate for this criteria. Storm sewers and di are major collectors, should be sized for a larger design storm such as the 25-year s

STORMWATER MANAGEMENT ALTERNATIVES

- ently, Lost Canyon Watershed is only approximately 15 percent developed. This ever is projected to be mostly developed in the near future. Current drainage and ty problems currently exist. It is therefore recommended that the facilities in Figure lled to help alleviate these potentially hazardous future stormwater problems, protein
- r quality of Lake Delton, and help to create a safe and aesthetically pleasing area.
- ne recommended facilities are multi-purpose water quality, drainage and flood co tures. Recommended alternatives are listed below: Regional Detention - Located in the infield of the southwest quadrant of I-9 1. Interchange and south of the interchange. Purpose is to alleviate the impact of the stormwater flows originating south of the interstate. This basin would requ maximum storage of 48 acre-ft. in order to attenuate the post-development 100
- should be addressed by constructing Best Management practices designed for development or group of developments (Area Controls). Conveyance - Open channel to convey 100-year stormwater north from the inte

storm from runoff conditions to that of pre-development (existing) conditions. recommended that this facility be designed for flood control only. Stormwater quantum sto

- along the west side of STH 12 to new culvert south of Aire Vista Motel and we existing greenway. In order to convey the future conditions, a trapezoidal gr swale would be required with a bottom width of 5 feet and a depth of 5 feet wit side slopes. These are the minimum dimensions of the primary drainage corrido the Lost Canyon Watershed. As a grassed drainage corridor, there is the advanta the channel functioning to filter and settle pollutants carried by stormwater runo
- Regional Detention Located east of STH 12. The purpose is to alleviate the ir of stormwater from future development originating in the northeast quadrant of I-9 Interchange east to Bunker Road. These basins (-7, -8 and -12) would requ
- in pollutant loadings are predicted from future development in this drainage area. facility can be designed as a regional water quality and flood control detention b An alternative would be to design the facility for flood control only. However Village would then have to adopt the policy requiring each development in the dra basin to construct their own facility to treat the "First Flush".

Pagional Detention I contad court of Progressive Drive in switting a

maximum storage of 75 acre-ft. in order to attenuate the 100-year storm to a flow can be safely handled by the 60" CMP under STH 12 can convey. Significant inc

- Detention/Channel and Culvert Improvements West side of STH 12 to Progre Drive. The purpose of this facility would be to supplement the storage of the 1
- regional facility south of Progressive Drive. This facility is too close to the well Progressive Drive to be suitable as a water quality facility.

Detention Dam - Located east of Highway 12 in Lost Canyon. The main purpo

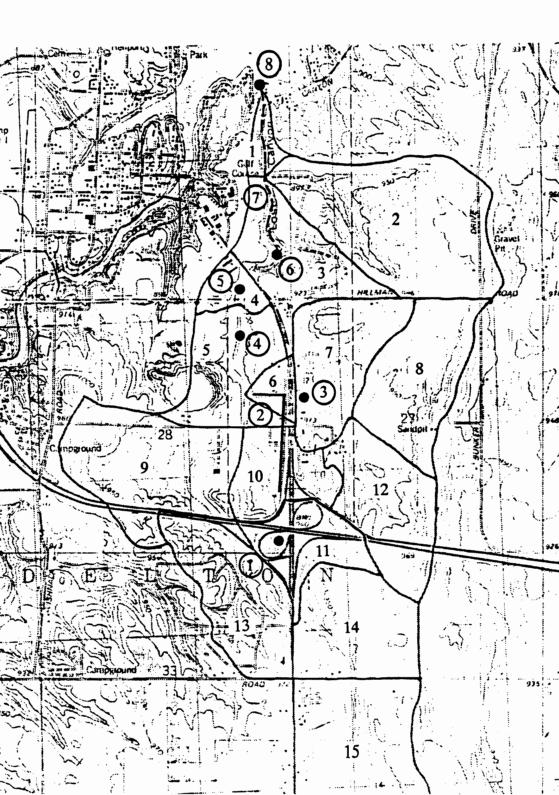
- this facility is protecting the water quality in Lake Delton by reducing the sedi transported through the canyon.

 Channel Stabilization Located north of the detention dam described in item 6 a
- Channel Stabilization Located north of the detention dam described in item 6 a and continuing north to Lake Delton. Purpose is to reduce erosion and seditransport to the Lake.
- B. Dredging Lost Canyon Bay. Purpose is to remove accumulated sediments. Pos maintenance dredging.
 Existing 42" CMP on the upstream side of STH 12, north of Progressive Drive, is grasized. It would be unrealistic for the Village to construct the required 190 acre-ft reg

tion basin (Recommendation -4) to prevent predicted flooding and overtopping of STF

solution should include the following elements:

- regional detention; increase the private culvert capacity leading to the box culvert under STH 112; channel stabilization in Lost Canyon;
- development of a site plan for the area between Progressive Drive and Gasser R and floodproofing of the concessionaire in Lost Canyon.
- ry preliminary estimate of costs for these solutions could range from \$1,000,000,000,000. Coordination and negotiation with the affected landowners, the Village and along with the required engineering goes beyond the limits of this investigation. However a critical issue which must be addressed by the Village as soon as possible.



IMPLEMENTATION APPROACH

sts for the recommended alternatives are presented in Table 7. Estimated costs inclu cent for engineering and 25 percent for contingencies. Land acquisition is included st estimates. It was assumed that the average per acre cost for land is \$8,000. Constr ould be phased to coincide with development and availability of funding.

PRIORITY WATERSHED

COSTS

ak County has filed an application with the DNR requesting that the Dell Creek Water included in the States Priority Watershed Program. Based on verbal comments fro VR, there is reason to be optimistic that the Dell Creek Watershed will be selected. Assi watershed is selected this year (1994), the Village may be able to apply for grants st 1995 to assist with the implementation of facilities that are designed to improve the ality of stormwater runoff from existing development. Total amount of the grants the lage may be eligible for is approximately \$250,000 to \$400,000.

PROJECT PRIORITY

WDB)-3 and -4. SWDB-3 is in a subbasin for which a major development is planned. important to have the necessary infrastructure in place to mitigate the impact of incioff volumes and added pollutant loadings. SWDB-4 is a partially constructed facility tical for storing and treating stormwater runoff from commercial lands and the business

commendations -5, -6 and -7 are also eligible for partial funding through DNR's Pr tershed Program. However, their timing is not critical. Construction of SWDB-1 shows ed to coincide with the development of Subbasins 14 and 15. Construction of ommended channel improvement (Recommendation - 2) should be completed at the same the reconstruction of the traffic alignment of the frontage roads. Table 8 summarize

e two most critical recommendations identified in Figure 6 are stormwater detention

gested priorities of the recommended improvements. IMPLEMENTATION PLAN

suggested implementation schedule is presented in Table 9. The schedule covers on rs of 1994 through 1996. This schedule and the priority for implementing the remains ommendations may change as development occurs.

Estimated Project Costs

Estimated Cos

\$3,947,000

1.	I-90/94 Detention Basin	\$1,045,000
2.	Channel Reconstruction	730,000
3.	Regional Detention Cost w/STH 12	1,800,000
4.	Regional Detention south of Progressive Drive	N/A
5 .	Detention north of Progressive Drive	150,000
6.	Lost Canyon Detention Dam	160,000
7.	Lost Canyon Channel Stabilization	22,000
8.	Dredging Lost Canyon Bay	20,000
9.	Annual Maintenance Dredging	20,000

Description

From Figure 6

oject <u>ımber*</u>

Total

A Cost estimates not available

Suggested Priority Recommended Improvements

	Project	
<u>iority</u>	Number*	Project
1	9	Lost Canyon Bay Dredging
2	3	Regional Detention East of Highway 12
3	4	Regional Detention South of Progressive D
4	5	Detention North of Progressive Drive
5	6	Lost Canyon Detention Dam
6	7	Lost Canyon Channel Stabilization
7	8	Maintenance Dredging
8	2	Channel Reconstruction
9	1	I-90/94 Detention Basin

From Figure 6

Suggested Implementation Plan

94

Implementation of stormwater utility billing system.

Adopt ordinance establishing a system of impact fees and special assessments. Preliminary design of the detention basin and greenway corridor south of Progressive

Adopt zoning regulations to set aside land for regional detention basins.

Apply for Dell Creek Watershed designation as a priority watershed.

Complete dredging of Lost Canyon Bay.

95

Preliminary design for detention basin east of Highway 12 (SWDA-3).

Modify zoning to include detention basin and greenway corridor in industrial park so Progressive Drive.

Acquire land for SWDA-4.

Complete design plans and specifications for SWDA-4.

96

Construction of SWDA-4.

Prepare plans and specifications for SWDA-3.

Acquire land for SWDA-3.

Acquire land for SWDA-5.

Recommendations 3 and 4

ecommendation No. 3 - SWDA east of Highway 12

Land acquired through dedication by developers.

50% special assessment based on a per acre basis.

50% impact fees based on impact fees.

User Fees/General Fund will front revenue that will eventually be collected by assessments and impact fees.

commendation No. 4 - SWDA South of Progressive Drive

50% DNR Local Assistance Grant.

assessments and impact fees.

25% special assessment based on a per acre basis.

25% impact fees based on impervious area.

User Fees/General Fund will front revenue that will eventually be collected by

REFERENCES

- JR, Wisconsin Construction Site Best Management Practice Handbook, 1989.
- nderwalle and Associates, Interstate 90/94 U.S. HWY 12 Interchange Land Use and equilation Study, 1991.

S. Department of Agriculture, Soil conservation Service, Sauk County Soil Survey, 19

oodward-Clyde Consultants, Lake Management Plan for Lake Delton, Sauk County, Wise